STRUCTURAL NOTES:

1. GENERAL
- 1.1. COORDINATE ALL INFORMATION CONTAINED IN THIS STRUCTURAL SET WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND OTHER TRADES. CONTACT BTK ENGINEERING IF CONFLICT IS FOUND.
- 1.2. SEE ARCHITECTURAL DRAWINGS FOR FINISHES.
- 1.3. REVIEW OF SUBMITTALS AND/OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
- 1.4. SHOP DRAWINGS REQUIRED:
  - 1.4.1. CONCRETE REINFORCEMENT/EMBEDS FABRICATION DRAWINGS
  - 1.4.2. STRUCTURAL STEEL - FABRICATION/ERECTION DRAWINGS
  - 1.4.3. STAIRS - FABRICATION/ERECTION DRAWINGS
  - 1.4.4. LIGHT GAUGE METAL STUD FRAMING (EXTERIOR) - ENGINEERED
  - 1.4.5. METAL BUILDING - ENGINEERED SYSTEM.
  - 1.4.6. MASONRY - PRODUCT DATA AND REINFORCEMENT
- 1.5. SUBMITTALS REQUIRED:
  - 1.5.3. SOILS COMPACTION REPORTS.
  - 1.5.4. CONCRETE MIX DESIGN.
  - 1.5.5. MORTAR MIX DESIGN
  - 1.5.6. CONCRETE TEST REPORTS.
  - 1.5.7. MASONRY UNIT - PRODUCT DATA
  - 1.5.8. METAL DECK - PRODUCT DATA
- 1.6. CONTRACTOR IS RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
- 1.7. SITE AND CONSTRUCTION SHALL COMPLY WITH OSHA OR EM385 AT ALL TIMES.
- 1.8. SITE SHALL BE MAINTAINED IN A CLEAN, ORDERLY, AND SAFE MANNER AT ALL TIMES.

2. SOILS
- 2.1. CONTRACTOR SHALL PROVIDE A GEOTECHNICAL REPORT WITH A MINIMUM OF 4 SOIL BORING TO A MINIMUM DEPTH OF 25' INCLUDING SPT WITH BLOW COUNT NUMBERS.
- 2.2. CONTRACTOR SHALL VERIFY SOIL IS FREE OF MUCK, CLAY, SILT, ORGANICS, OR OTHER UNSUITABLE MATERIALS.
- 2.3. CONTRACTOR SHALL REMOVE ALL LAYERS OF SOIL THAT CONTAIN ORGANICS.
- 2.4. CONTRACTOR SHALL VERIFY FLOOD ZONES AND WATER TABLES AND ASSURE FINISH FLOOR IS AT THE REQUIRED ELEVATION.
- 2.5. CONTRACTOR SHALL VERIFY AND COMPLY WITH ALL BUILDING SETBACKS AND EASEMENTS.
- 2.6. SOIL SHALL BE CAPABLE OF SUPPORTING AND ALLOWABLE BEARING PRESSURE OF 2000 PSF.
- 2.7. CONTRACTOR SHALL VERIFY ALL SOILS ARE COMPACTED TO 98% MAXIMUM DENSITY (MODIFIED PROCTOR).
- 2.8. ALL SOILS UNDER SLABS SHALL BE TREATED FOR TERMITES.
- 2.9. STRUCTURAL BACKFILL AND FILL SOILS
  - 2.9.1. COMPLY WITH GEOTECHNICAL REPORT FOR STRUCTURAL FILL OR FILL REQUIRED FOR SITE DEVELOPMENT. THIS SHOULD BE PLACED IN LOOSE LIFTS NOT EXCEEDING 12 INCHES IN THICKNESS WHEN COMPACTED BY THE USE OF A VIBRATORY DRUM ROLLER. THE LIFT THICKNESS SHOULD BE REDUCED TO 8 INCHES IF THE ROLLER OPERATES IN THE STATIC MODE OR IF TRACK-MOUNTED COMPACTION EQUIPMENT IS USED. IF HAND-HELD COMPACTION EQUIPMENT IS USED, THE LIFT THICKNESS SHOULD BE FURTHER REDUCED TO 6 INCHES. STRUCTURAL FILL IS DEFINED AS A NON-PLASTIC, INORGANIC, GRANULAR SOIL HAVING LESS THAN 10 PERCENT MATERIAL PASSING THE NO. 200 MESH SIEVE AND CONTAINING LESS THAN 4 PERCENT ORGANIC MATERIAL. TYPICALLY, THE MATERIAL SHOULD EXHIBIT MOISTURE CONTENTS WITHIN ±2 PERCENT OF THE MODIFIED PROCTOR OPTIMUM MOISTURE CONTENT (ASTM D 1557) DURING THE COMPACTION OPERATIONS. COMPACTION SHOULD CONTINUE UNTIL DENSITIES OF AT LEAST 98 PERCENT OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D 1557) HAVE BEEN ACHIEVED WITHIN EACH LIFT OF THE COMPACTED STRUCTURAL FILL.

3. CONCRETE
- 3.1. FOOTINGS SHOWN IN THIS SET OF DRAWINGS NOT FINAL SUBJECT TO CHANGE PENDING THE METAL BUILDING SUPPORT REACTIONS AND THE GEOTECHNICAL REPORT. NO CONSTRUCTION MAY TAKE PLACE UNTIL THESE TWO SUBMITTALS HAVE BEEN REVIEWED AND ANY ADJUSTMENT MADE TO THESE DRAWINGS.
- 3.2. CAST IN PLACE CONCRETE SHALL BE IN ACCORDANCE WITH ACI 318-14.
- 3.3. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM COMPRESSIVE BREAK STRENGTH AFTER 28 DAYS.
  - 3.3.1. COLUMN FOOTINGS 3000 PSI
  - 3.3.2. WALL FOOTINGS 4000 PSI
  - 3.3.3. BOND BEAMS AND HEADERS 4000 PSI 450 PSI FLEXURAL
  - 3.3.4. SLAB 4500 PSI 500 PSI FLEXURAL
- 3.4. CONCRETE MIX DESIGN SHALL BE SUBMITTED TO BTK ENGINEERING FOR APPROVAL PRIOR TO PROCUREMENT. ALLOW ONE WEEK FOR REVIEW.
- 3.5. CONCRETE SHALL HAVE FIELD CYLINDERS TAKEN AND TESTED IN ACCORDANCE WITH ACI 318.
- 3.6. CONCRETE SLUMP SHALL BE BETWEEN 3 AND 6 INCHES AT THE TIME OF PLACEMENT.
- 3.7. CONCRETE COVER SHALL BE IN ACCORDANCE WITH SECTION 7.7.1, ACI318-14:

  CONCRETE EXPOSED TO EARTH OR WEATHER
  #6 THROUGH #18 BARS 2"
  #5 BAR W31 OR D31 WIRE OR SMALLER 1 ½"

  CONCRETE NOT EXPOSED TO EARTH OR WEATHER
  #11 BARS OR SMALLER ¾"

  FOOTINGS AND GRADE BEAMS SHALL HAVE 3" REGARDLESS OF THE BAR SIZE OR THE DIRECTION TO THE EDGE.
- 3.8. ALL FOUNDATION REINFORCING BARS SHALL BE GRADE 60, ASTM616 AND LAP 36 BAR DIAMETERS.
- 3.9. ALL CAST IN PLACE NOT ASSOCIATED WITH THE FOUNDATION SHALL BE GRADE 60, ASTM615 AND HAVE A CLASS B TENSION LAP SPLICE
- 3.10. WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A185 AND LAP A MINIMUM OF 6".
- 3.11. WELDED WIRE REINFORCEMENT SHALL BE PLACED IN THE UPPER ONE HALF OF THE SLAB ON SUPPORTS (NOT PULLED INTO PLACE.)
- 3.12. PROPORTION NORMAL-WEIGHT CONCRETE MIXTURE AS FOLLOWS:
  - 3.12.1. PORTLAND CEMENT: ASTM C 150, TYPE I/II, **NO FLY ASH PERMITTED**
  - 3.12.2. MINIMUM COMPRESSIVE STRENGTH: 3000, 4000, AND 4500 PSI AT 28 DAYS.
  - 3.12.3. MAXIMUM WATER-CEMENTITIOUS MATERIALS RATIO: 0.51.
  - 3.12.4. SLUMP LIMIT: 3" TO 6".
  - 3.12.5. NORMAL-WEIGHT AGGREGATES: ASTM C 33, CLASS 3M COARSE AGGREGATE OR BETTER, GRADED.
  - 3.12.6. MAXIMUM COARSE-AGGREGATE SIZE: 3/4" MAXIMUM UNLESS NOTED.
  - 3.12.7. FINE AGGREGATE: FREE OF MATERIALS WITH DELETERIOUS REACTIVITY TO ALKALI IN CEMENT.
  - 3.12.8. AIR CONTENT: 4 PERCENT, PLUS OR MINUS 1.5 PERCENT AT POINT OF DELIVERY FOR 3/4-INCH (38-MM) NOMINAL MAXIMUM AGGREGATE SIZE.
  - 3.12.9. NO CALCIUM CHLORIDE PERMITTED.
  - 3.12.10. HIGH EARLY SET ADMIXTURES ARE ENCOURAGED IF THEY ARE NON CORROSIVE TO THE REINFORCEMENT.
- 3.13. FINISH TEXTURE SHALL BE VERIFIED WITH ARCHITECT.
- 3.14. CONCRETE SURFACE SHALL BE UNIFORM AND STRAIGHT AND LEVEL TO WITHIN 1/8" IN A TEN FOOT STRAIGHT EDGE.

4. MASONRY
- 4.1. ALL CMU BELOW FINISH FLOOR SHALL BE POURED SOLID WITH 3000 PSI GROUT CONFORMING TO ASTM C476.
- 4.2. CONCRETE MASONRY WORK SHALL CONFORM TO ACI 530, BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES AND ACI 530.1 SPECIFICATION FOR MASONRY STRUCTURES.
- 4.3. CONCRETE MASONRY SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1,500 PSI.
- 4.4. MORTAR SHALL COMPLY WITH THE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY AND SHALL BE.
  - 4.4.1. WALLS BELOW GRADE TYPE M
  - 4.4.2. WALLS ABOVE GRADE TYPE S
- 4.5. REINFORCED CONCRETE MASONRY UNITS SHALL BE GROUTED WITH 3,000 PSI COURSE GROUT CONFORMING TO ASTM C476.
- 4.6. WALL HORIZONTAL REINFORCEMENT SHALL BE 9 GA TRUSS TYPE AT 16" O/C.
- 4.7. ALL WALL REINFORCEMENT SHALL BE LAPPED A MINIMUM OF 48 BAR DIAMETER.
- 4.8. MASONRY CONTROL JOINTS SHALL BE LOCATED BY ARCHITECT AT NATURAL BREAKS OR BENDS IN THE STRUCTURE AND 20'-0" O/C MAX.

5. STRUCTURAL STEEL
- 5.1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED ACCORDING TO AISC 360-10 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS."
- 5.2. STEEL FABRICATOR'S SPECIALTY ENGINEER SHALL DESIGN ANY CONNECTIONS NOT DETAILED IN THESE DOCUMENTS. THE SPECIALTY ENGINEER SHALL BE REGISTERED IN THE PROJECT STATE. CONNECTION DESIGN CALCULATIONS AND STEEL DETAILER'S SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE SPECIALTY ENGINEER AND SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. SUBMIT SHOP DRAWINGS PREPARED IN ACCORDANCE WITH AISC MANUAL "DETAILING FOR STEEL CONSTRUCTION", LATEST EDITION. STEEL
- 5.3. STRUCTURAL STEEL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, 50 KSI. STRUCTURAL STEEL SHAPES, PLATES, ANGLES, AND CHANNELS SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE. STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI, UNLESS NOTED OTHERWISE. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554-07a GRADE 36 UNLESS NOTED OTHERWISE.
- 5.4. BOLTS SHALL CONFORM TO ASTM A325, TYPE 3 (CORROSION RESISTANCE), ¾-INCH DIAMETER MINIMUM, UNLESS NOTED OTHERWISE. BOLTS IN BEARING CONNECTIONS SHALL BE DESIGNATED TYPE N, TENSIONED, SNUG-TIGHT AS DEFINED BY AISC. ALL OTHER BOLTS SHALL BE PRE-TENSIONED.
- 5.5. USE PRE-QUALIFIED WELDED JOINTS AS PER AISC, AND AWS D1.1 "STRUCTURAL WELDING CODE." USE ONLY CERTIFIED WELDERS; ALL ELECTRODES SHALL CONFORM TO AWS A5 GRADE E70XX. BARE ELECTRODE AND GRANULAR FLUX SHALL CONFORM TO AWS A5, F70 AWS FLUX CLASSIFICATION. MINIMUM WELD SIZE TO BE 3/16" FILLET WELD, U.N.O.
- 5.6. CUTS, BOLTS, COPING, ETC. REQUIRED FOR WORK OR OTHER TRADES SHALL BE SHOWN ON THE SHOP DRAWINGS AND MADE IN THE SHOP. CUTS OR BURNING HOLES IN STRUCTURAL STEEL MEMBERS IN THE FIELD WILL ONLY PERMITTED ON AN INDIVIDUAL, REVIEWED BASES.
- 5.7. SHOP CONNECTIONS NOT SPECIFICALLY DETAILED ON THE DRAWINGS MAY BE WELDED OR BOLTED. FIELD CONNECTIONS NOT SPECIFICALLY DETAILED ON THE DRAWINGS SHALL BE BOLTED, WHERE POSSIBLE.
- 5.8. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AND DRAWINGS RELATED TO OTHER TRADES. CONTRACTOR SHALL BE RESPONSIBLE TO CHECK AND COORDINATE DIMENSIONS, CLEARANCES, ETC. WITH THE WORK OF OTHER TRADES. THE STRUCTURAL STEEL CONTRACTOR SHALL PROVIDE FRAMING AROUND OPENINGS IN ROOF AS INDICATED IN THE MECHANICAL AND ARCHITECTURAL DRAWINGS.
- 5.9. STRUCTURAL STEEL CONTRACTOR SHALL COORDINATE THE BOTTOM OF BASE PLATE ELEVATION WITH THE TOP OF CONCRETE ELEVATION. IN CASE OF CONFLICT, THE CONTRACTOR SHALL MAKE ALLOWANCE IN HIS BID FOR MORE STRINGENT REQUIREMENTS.
- 5.10. STRUCTURAL STEEL SHALL BE PRIMED AND PAINTED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.
- 5.11. ALL STRUCTURAL STEEL FOR THIS PROJECT SHALL BE HOT DIPPED GALVANIZED MINIMUM OF G90 COATING.
- 5.12. ALL WELDS OR FIELD CUTTING AND FITTING SHALL BE GROUND CLEAN AND COATED WITH COLD APPLIED GALVANIZING.
- 5.13. ALL STRUCTURAL STEEL TO BE GROUNDED TO PROJECT ELECTRICAL GROUND.

EAVE HEIGHT	22.67'
ROOF SLOPE	3:12
COLLATERAL/GRAVITY	15 PSF
ROOF LIVE LOAD (REDUCIBLE)	20 PSF
FIRST FLOOR LIVE LOAD	100 PSF

TABLE OF CONTENTS

APPLICABLE CODES

Florida Building Code, Building (FBC-B)	2023
Minimum Design Loads For Building and Other Structures	ASCE 7-22

Cast in Place Reinforcement Lap Splices

			Lap splice			
Bar #	Dia	Foundation	Other Locations (f'c) Class B			
			3000	4000	4500	5000
	in	in	in	in	in	in
#3	0.375	12	21.4	18.5	17.4	16.5
#4	0.5	15	28.5	24.7	23.3	22.1
#5	0.625	18.75	35.6	30.8	29.1	27.6
#6	0.75	22.5	42.7	37.0	34.9	33.1

Cast in Place Hook Development Lengths

				90	180
Bar #	Dia	Ldh	radius	ext	ext
	in	in	in	in	in
#3	0.375	8.2	1.1	4.5	2.5
#4	0.5	11.0	1.5	6.0	2.5
#5	0.625	13.7	1.9	7.5	2.5
#6	0.75	16.4	2.3	9.0	3

CARLISLE BAPTIST CHURCH STRUCTURAL PHASE I.DWG

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Building Dimensions

Width	Length	Height	Slope	Angle
ft	ft	ft	on 12	Degrees
125.33	144	33	2	9.5

Calculating "a"

Least Horizontal Dimension	125.33 ft
Minimum of	
10% of Least Horizontal Dimension	12.533 ft
40% of Height	13.2 ft
But,	
Not less than 4% of Least Horizontal Dimension	5.0132 ft
Not Less than 3'-0"	3 ft
Therefore "a" =	12.533 ft

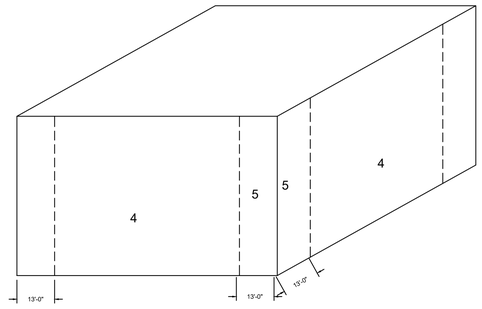
Calculating Base Pressure

Wind Velocity	140.00 mph
Risk Category	II
Exposure Category	C
Enclosure	Enclosed
ALFA	9.50
Zg	900.00 ft
Zstes	33.00 ft
Kz	1.00
Kd	0.85
Kzt	1.00
Base Pressure	42.74 psf

Component and Cladding Pressures (Effective Wind Area ≤ 10 sqft)

Zone	GCp	GCpi	qh psf	(GCp+GCpi)	qh[(GCp+GCpi)] psf (ult)	qh[(GCp+GCpi)] psf (asd)
Roof Zones+	0.6	0.18	42.74	0.78	33.34	20.00
1-	-2	-0.18	42.74	-2.18	-93.18	-55.91
2-	-2.7	-0.18	42.74	-2.88	-123.10	-73.86
3-	-3.6	-0.18	42.74	-3.78	-161.56	-96.94
4+	1	0.18	42.74	1.18	50.44	30.26
4-	-1.1	-0.18	42.74	-1.28	-54.71	-32.83
5+	1	0.18	42.74	1.18	50.44	30.26
5-	-1.4	-0.18	42.74	-1.58	-67.53	-40.52

NOTE: DESIGN PRESSURES FOR ALLOWABLE STRESS PRESSURES HAVE ALREADY BEEN REDUCED BY THE 0.6 FACTOR IN THE LOAD COMBINATIONS AS DEPICTED IN ASCE 7-22 SECTION 2.4. COMPONENT DESIGNER MAY NOT TAKE A 0.6 FACTOR AS A DOUBLE REDUCTION.



WALLS

WM-1 / S-2 WIND MAP
SCALE: 1/16"=1'

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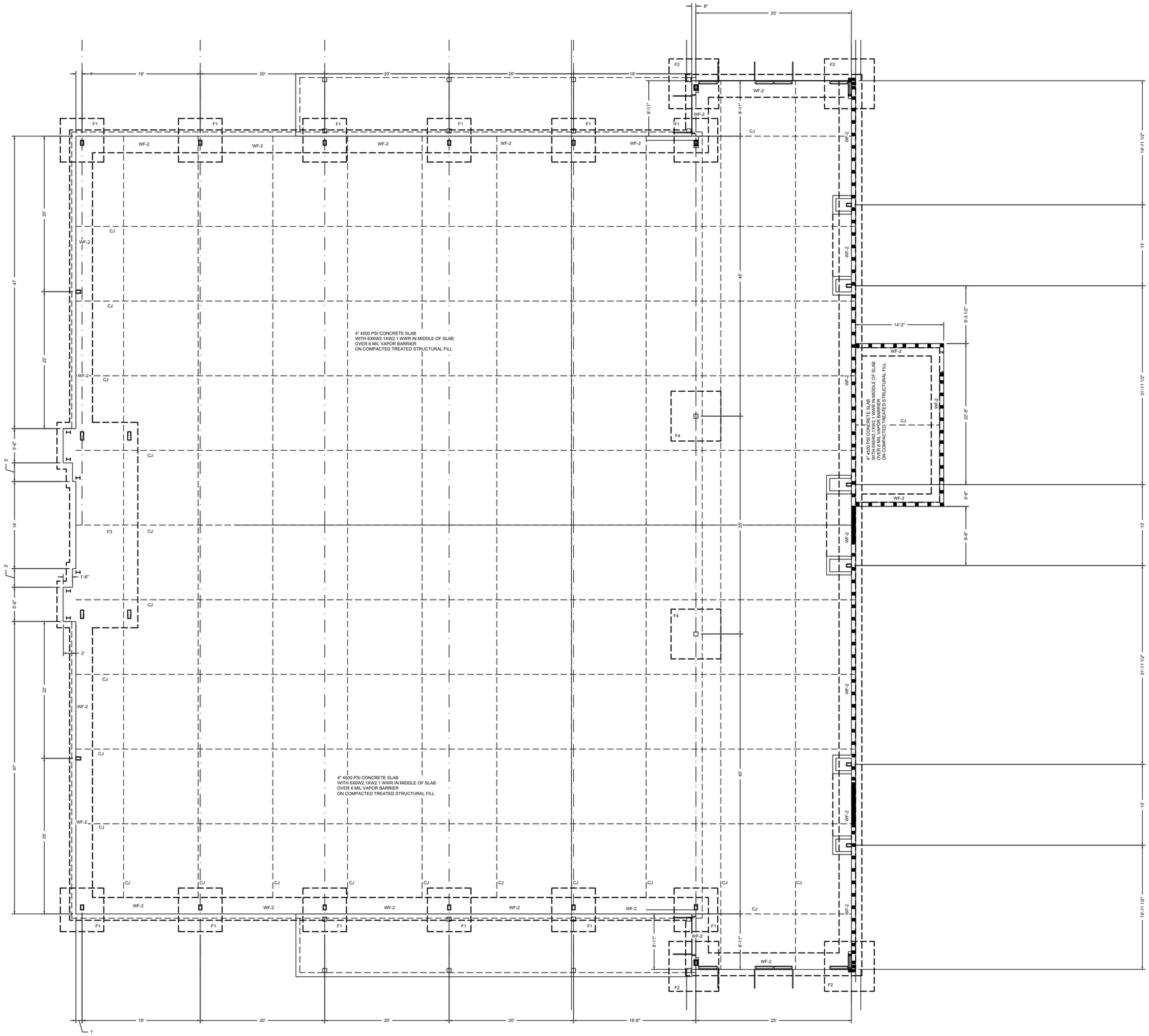

FD-1 / S-3 FOUNDATION PLAN
SCALE: 1/8" = 1'-0"

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BRADLEY TODD KENT P.E. #59384 STATE OF FLORIDA LICENSED PROFESSIONAL ENGINEER

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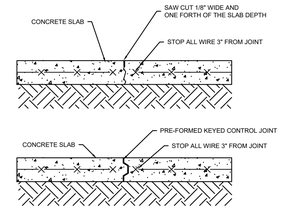
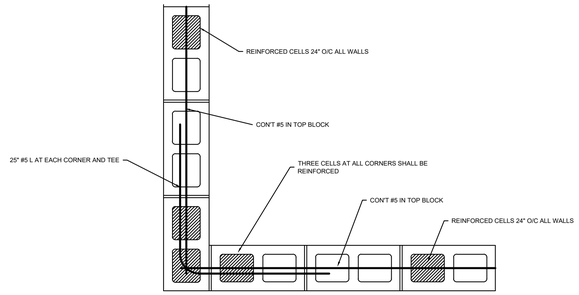
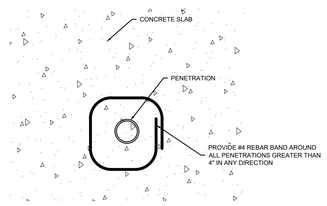


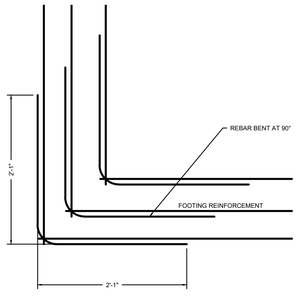
CJ-1 / S-4 CONTROL JOINT OPTIONS
SCALE: 1" = 1'-0"



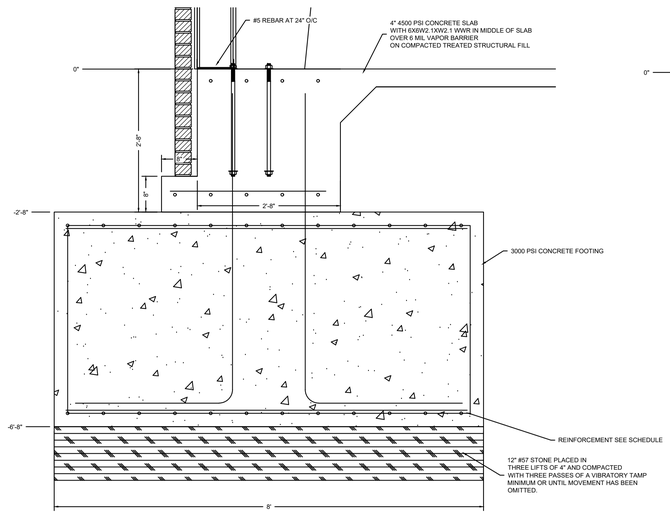
SWC / S-4 MASONRY CORNER REINFORCEMENT
SCALE: 1" = 1'-0"



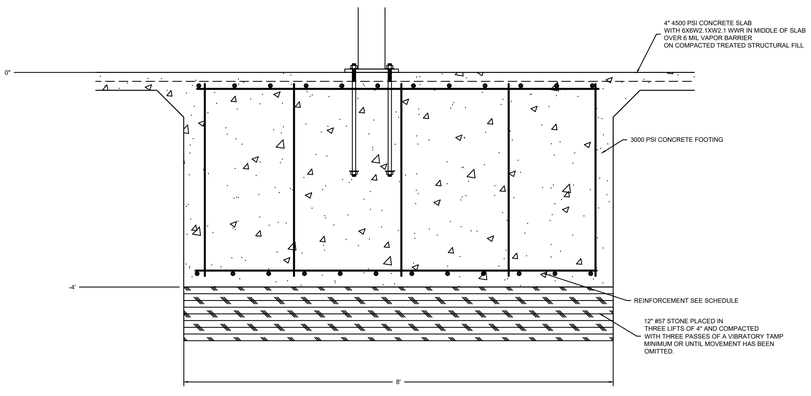
SP-1 / S-4 SLAB PENETRATION DETAIL
SCALE: 1" = 1'-0"



CR / S-4 CORNER REINFORCEMENT AT FOOTING
SCALE: 1" = 1'-0"



FD-X / S-4 TYPICAL FOOTING/WALL FOOTING DETAIL
SCALE: 3/4" = 1'-0"



FD-X / S-4 TYPICAL INTERIOR GRADE FOOTING DETAIL
SCALE: 3/4" = 1'-0"

Footing Schedule								
Mark	Depth	Length	Width	ELEVATION	Rebar			Anchor Bolts
					Top	Bottom	Sides	
F-1	4'-0"	7'-0"	7'-0"	-2'-8"	#6 REBAR 8" O/C EACH WAY	#6 REBAR 8" O/C EACH WAY	#6 REBAR (4) PER FACE	(4) 1 1/4" X 24"
F-2	4'-0"	8'-0"	8'-0"	-2'-8"	#6 REBAR 8" O/C EACH WAY	#6 REBAR 8" O/C EACH WAY	#6 REBAR (4) PER FACE	(4) 1 1/4" X 24"
F-3	2'-0"	33'-0"	13'-0"	0'-0"	#5 REBAR 12" O/C EACH WAY	#5 REBAR 12" O/C EACH WAY	N/A	(4)3/4" x 18"
F-4	4'-0"	8'-0"	8'-0"	0'-0"	#6 REBAR 8" O/C EACH WAY	#6 REBAR 8" O/C EACH WAY	#6 REBAR (4) PER FACE	(4) 1 1/4" X 24"
WF-1	1'-8"	CONT	1'-8"	0'-0"	#5 REBAR CONT	(3) #5 REBAR CONT	N/A	N/A
WF-2	2'-8"	CONT	2'-8"	0'-0"	#5 REBAR CONT AT 8" O/C	#5 REBAR CONT AT 8" O/C	#3 STIRRUP AT 16" O/C	N/A

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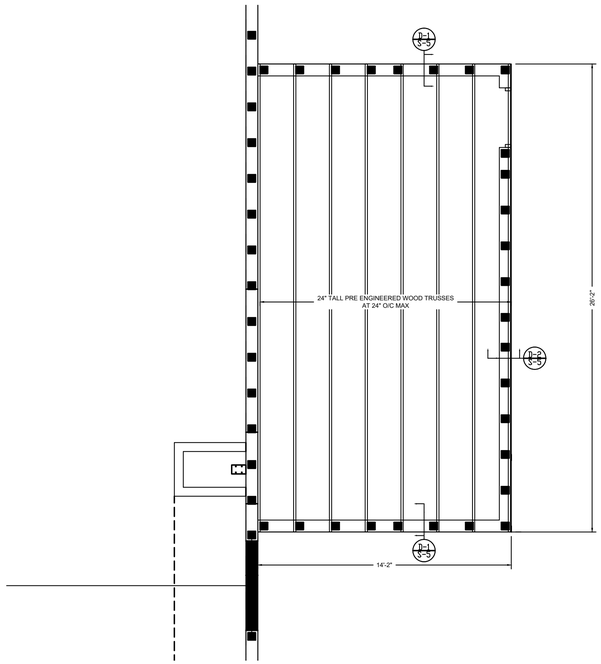
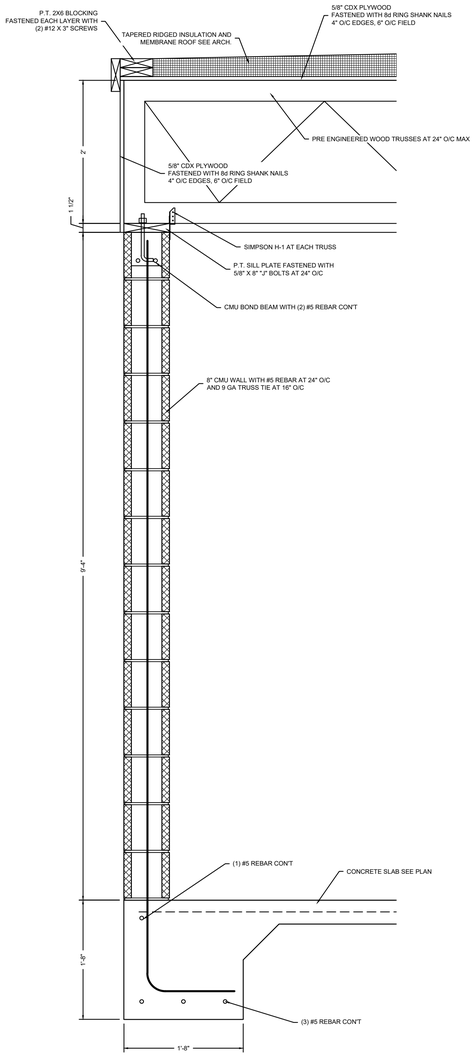
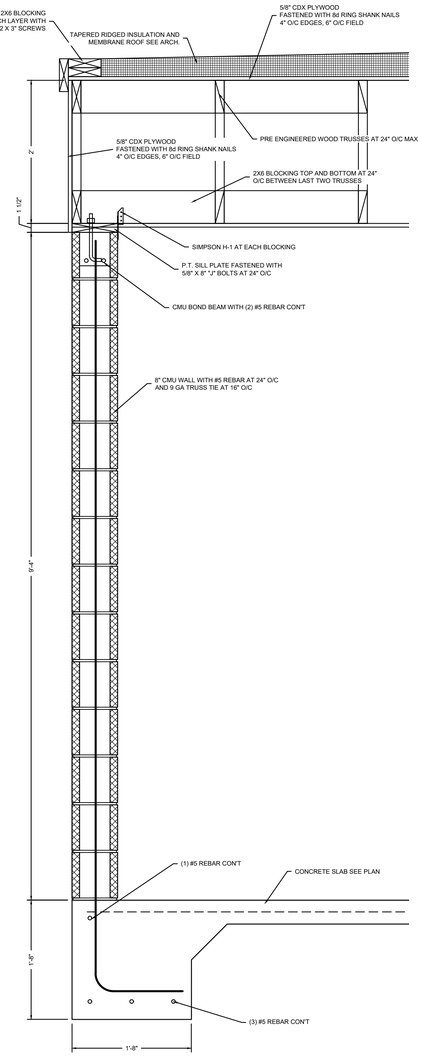


FR-1 / S-5 ACCESSORY ROOM ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"



D-1 / S-5 ACCESSORY ROOM STRUCTURAL WALL DETAIL
SCALE: 1" = 1'-0"



D-2 / S-5 ACCESSORY ROOM STRUCTURAL WALL DETAIL
SCALE: 1" = 1'-0"

CARLISLE BAPTIST CHURCH STRUCTURAL PHASE I.DWG

BRADLEY TODD KENT
LICENSED
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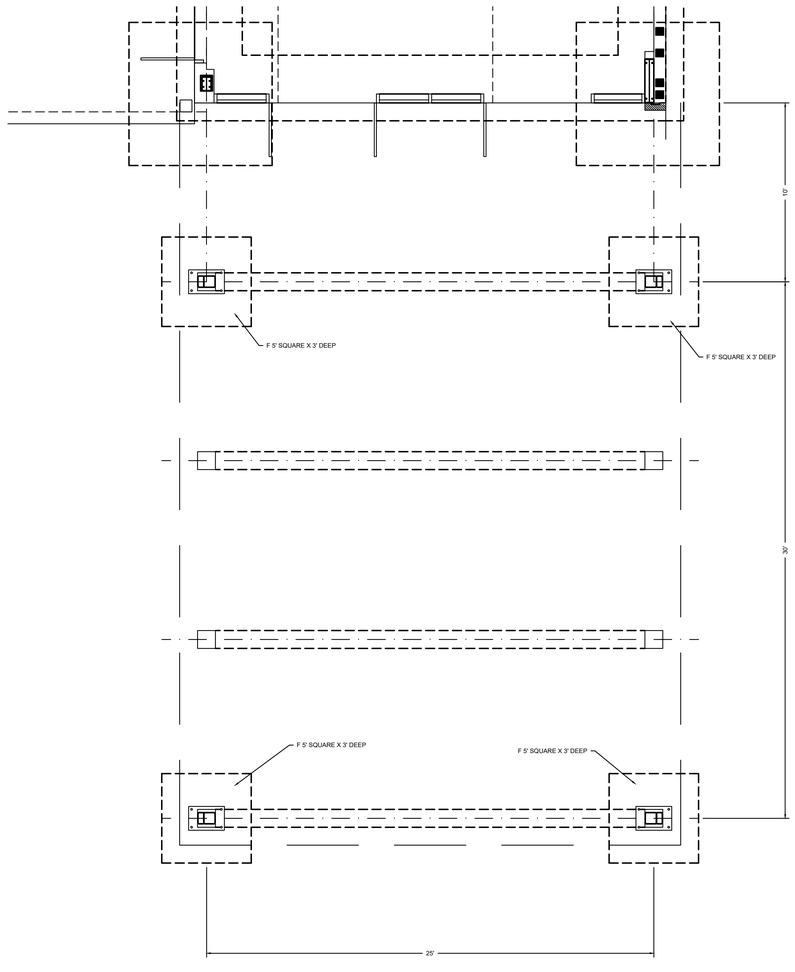
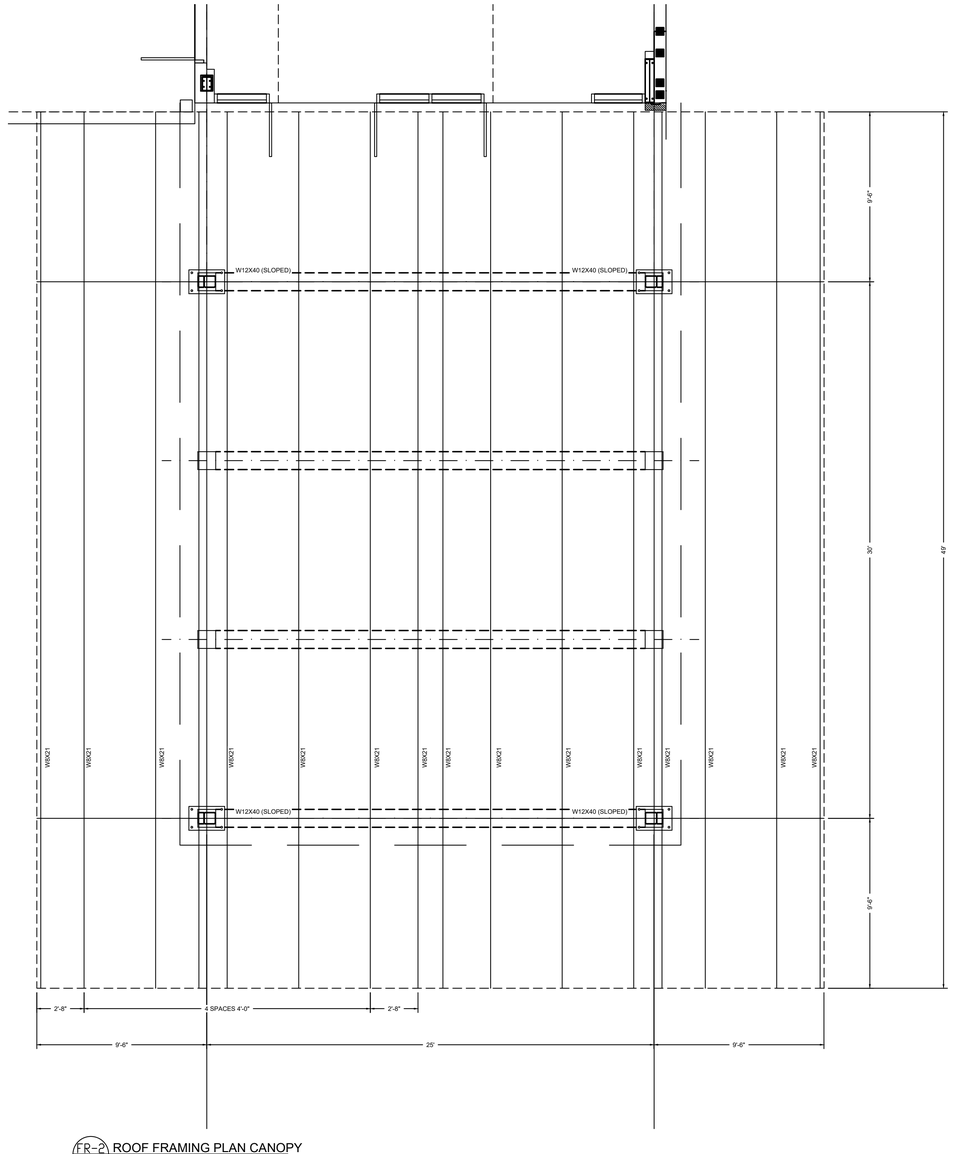


FD-2 / S-6 FOUNDATION PLAN CANOPY
SCALE: 1/4" = 1'-0"



FR-2 / S-6 ROOF FRAMING PLAN CANOPY
SCALE: 1/4" = 1'-0"

CARLISLE BAPTIST CHURCH STRUCTURAL PHASE I.DWG

BRADLEY TODD KENT STATE OF FLORIDA LICENSED ENGINEER

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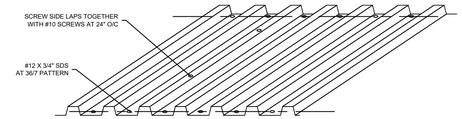
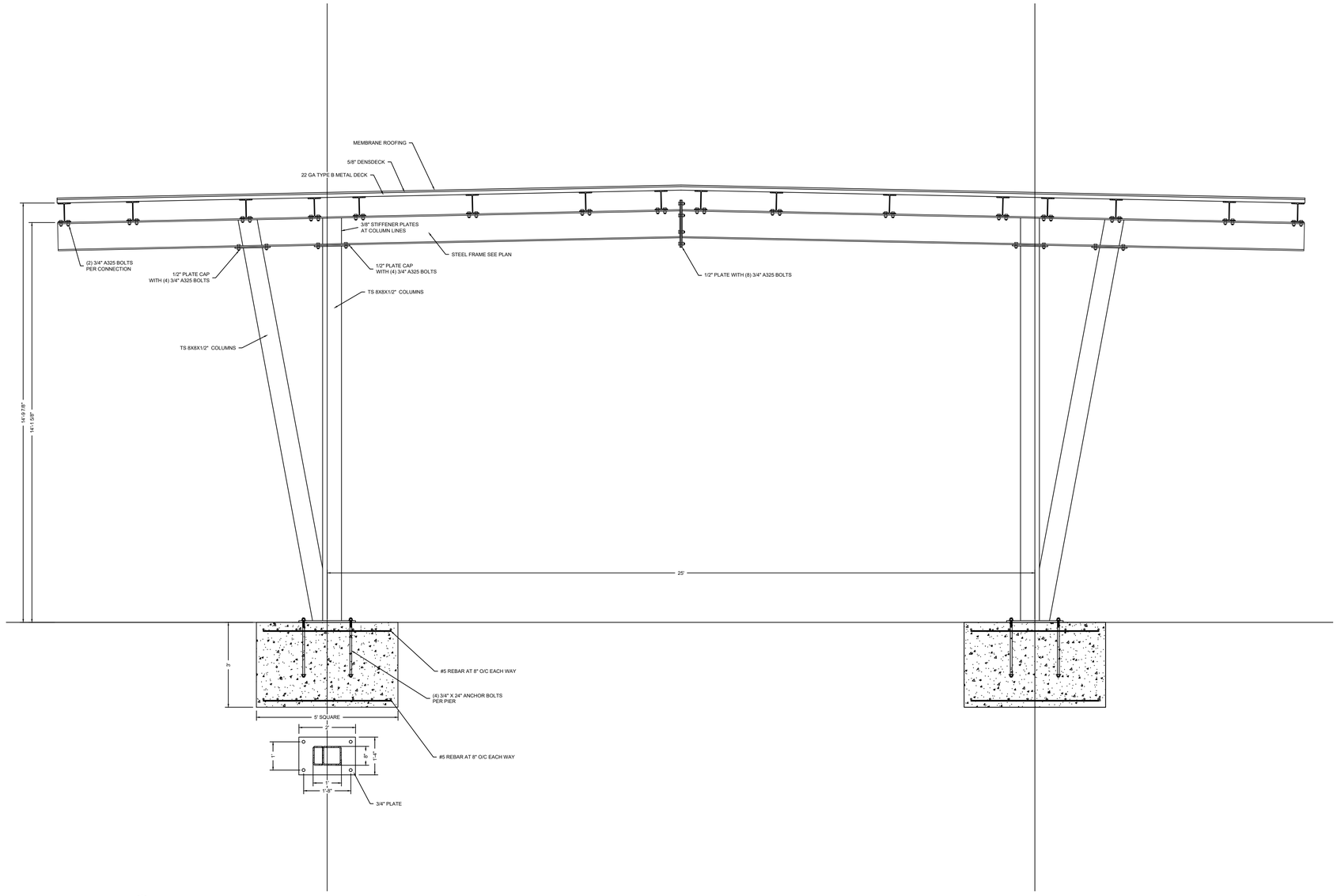


VULCRAFT 1.5B22 OR EQUAL

MD-1 / S-7 DECK FASTENING PATTERNS
SCALE: 1" = 1'-0"



SC-1 / S-7 CANOPY SECTION
SCALE: 1/2" = 1'-0"

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BRADLEY TODD KENT LICENSE

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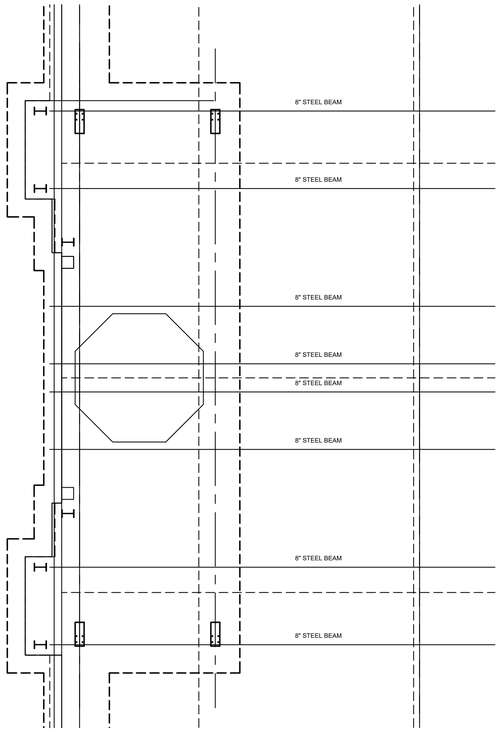
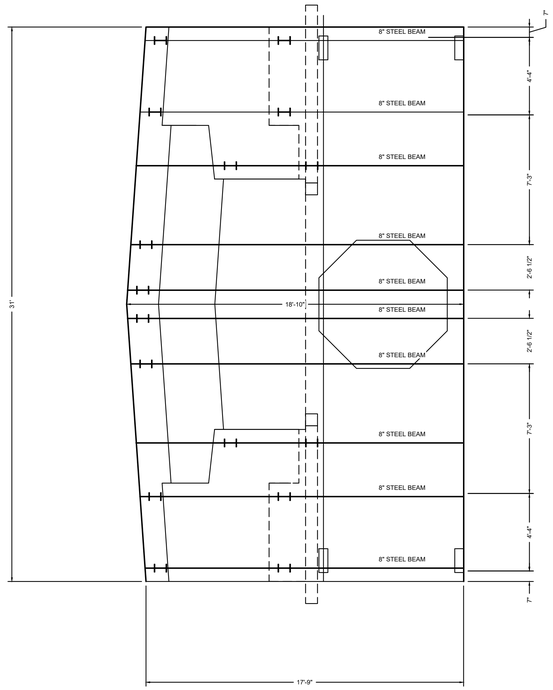
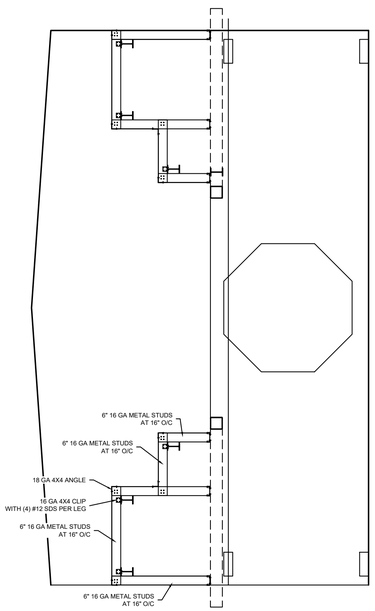


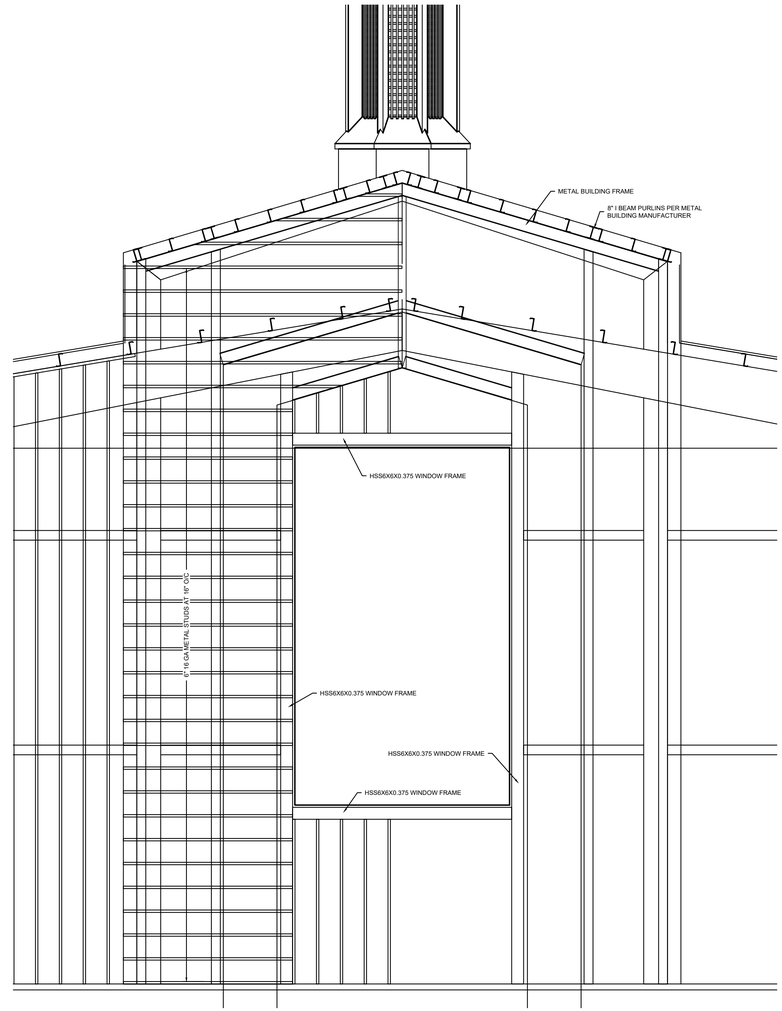
RF-3 / S-8 STEEPLE FOUNDATION PLAN
SCALE: 1/4" = 1'-0"



RF-3 / S-8 STEEPLE ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"



RF-3 / S-8 STEEPLE LIGHT GAUGE FRAMING PLAN
SCALE: 1/4" = 1'-0"



RF-4 / S-8 STEEPLE WALL FRAMING PLAN
SCALE: 1/4" = 1'-0"

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BRADLEY TODD KENT LICENSED PROFESSIONAL ENGINEER

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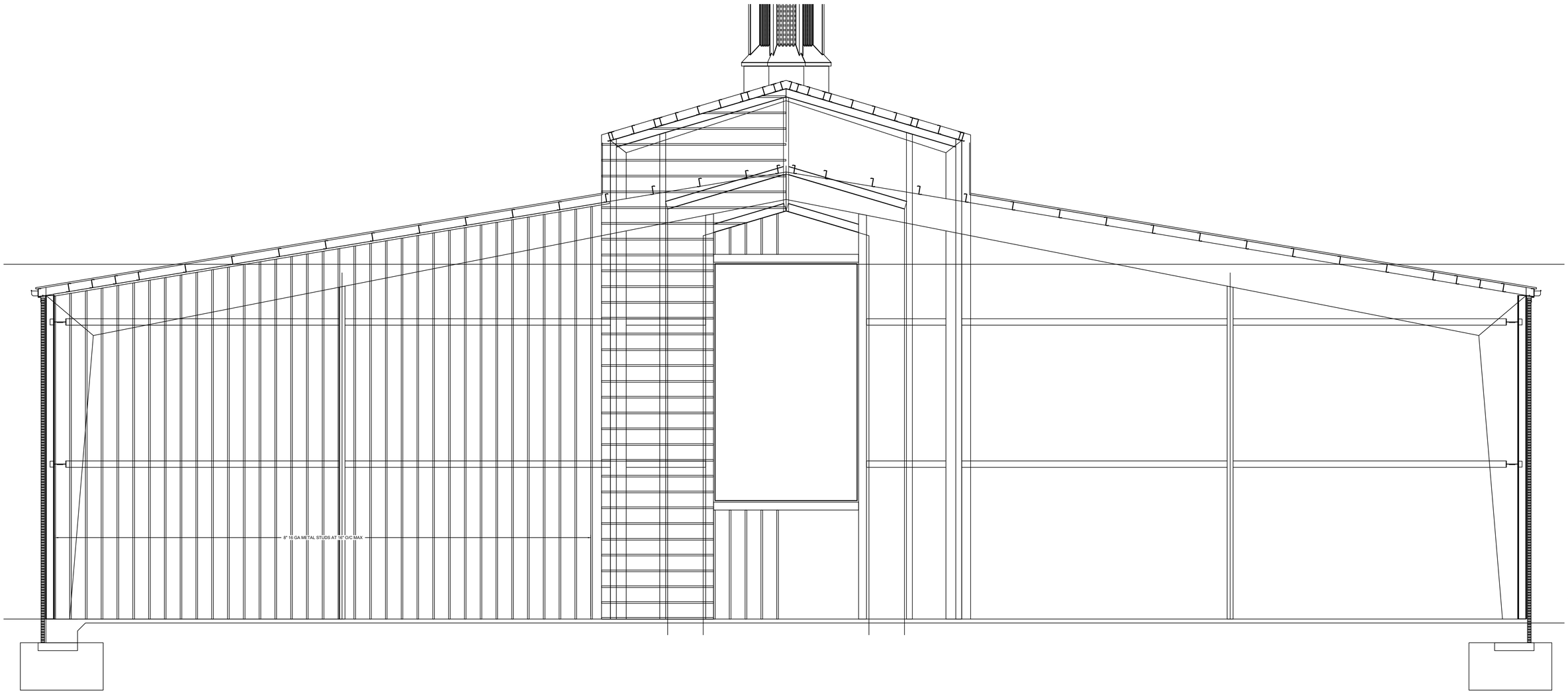

WF-1 / S-9 WALL FRAMING PLAN
SCALE: 1/4" = 1'-0"

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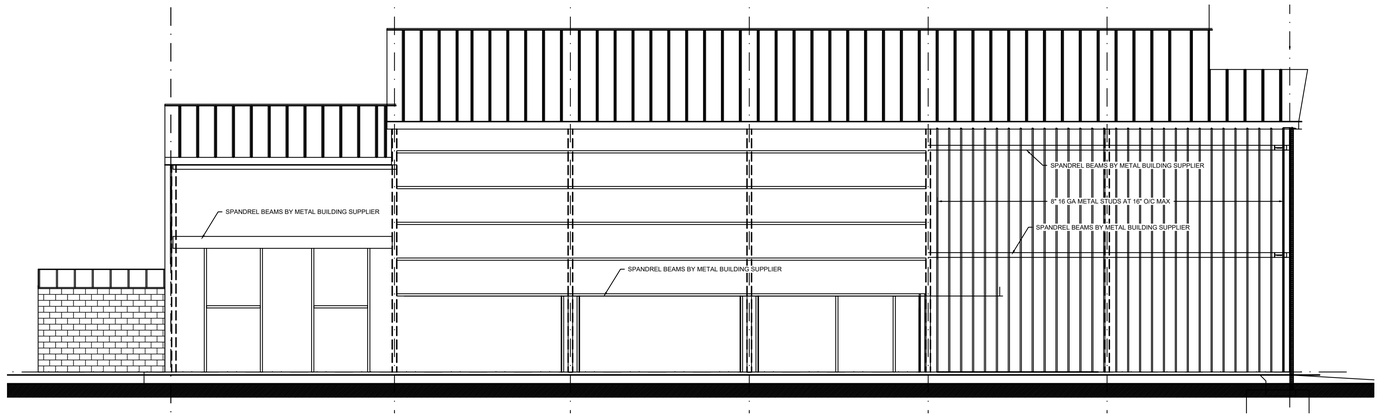
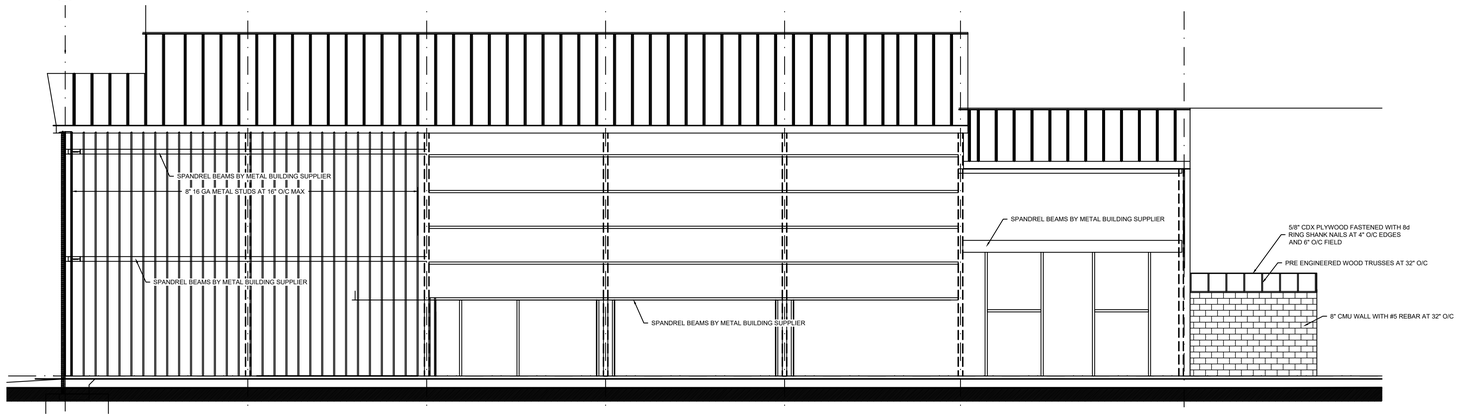
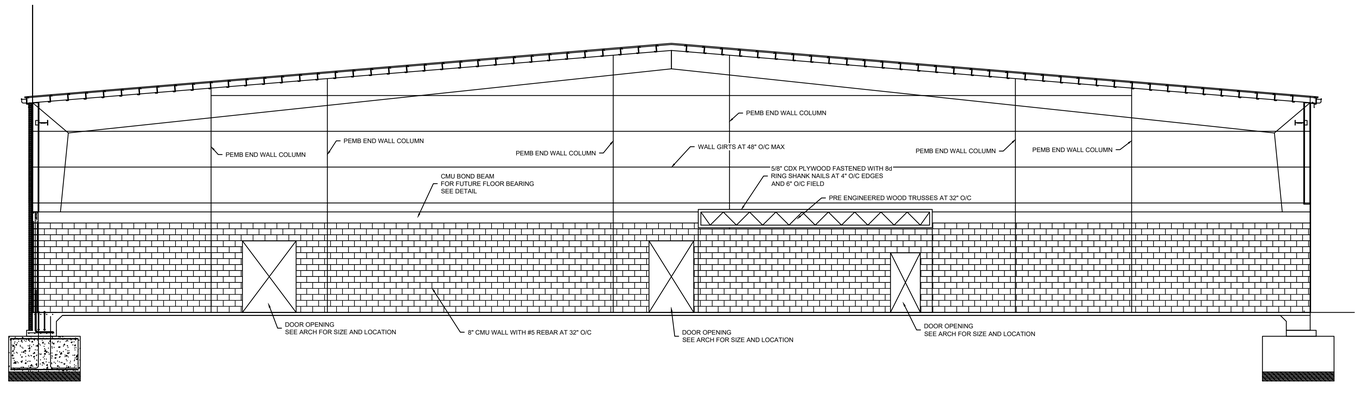


WF-2 / S-10 WALL FRAMING PLAN
SCALE: 1/8" = 1'-0"



WF-3 / S-10 WALL FRAMING PLAN
SCALE: 1/8" = 1'-0"



WF-4 / S-10 WALL FRAMING PLAN
SCALE: 1/8" = 1'-0"

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BRADLEY TODD KENT LICENSE

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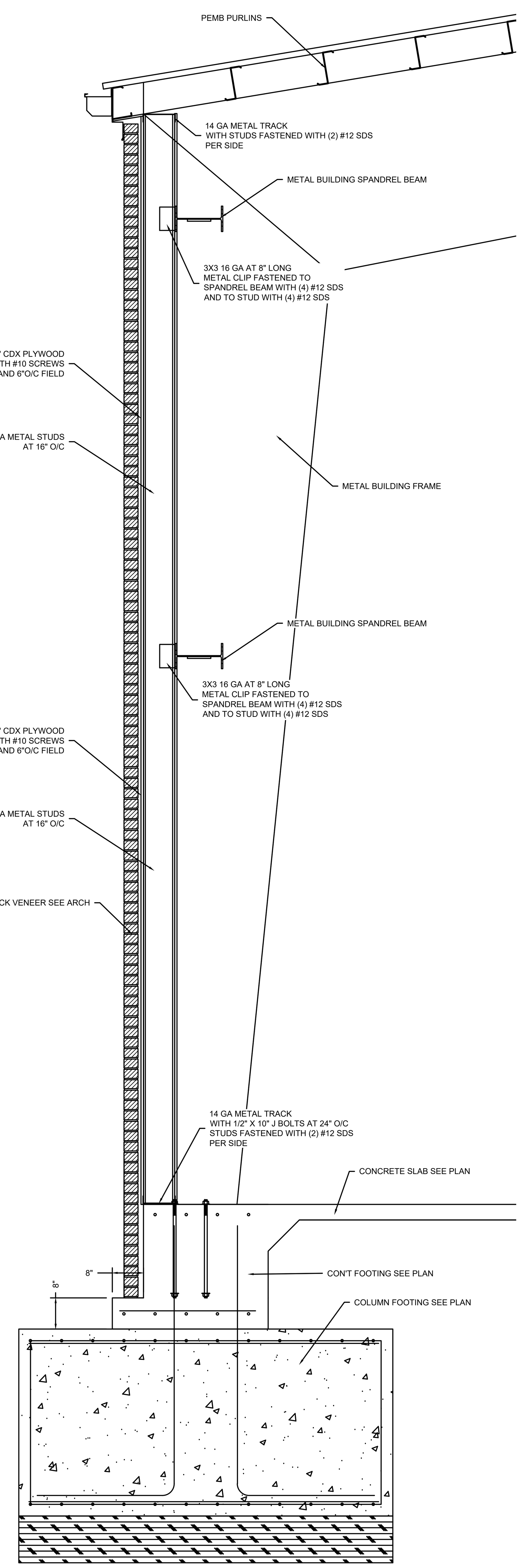
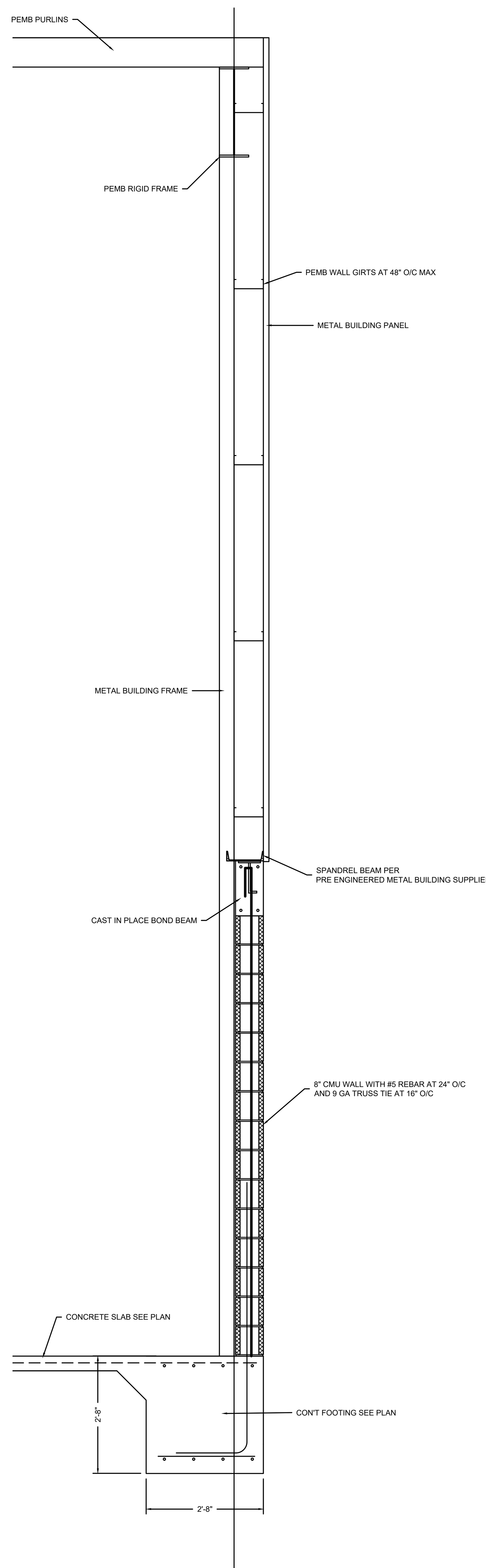
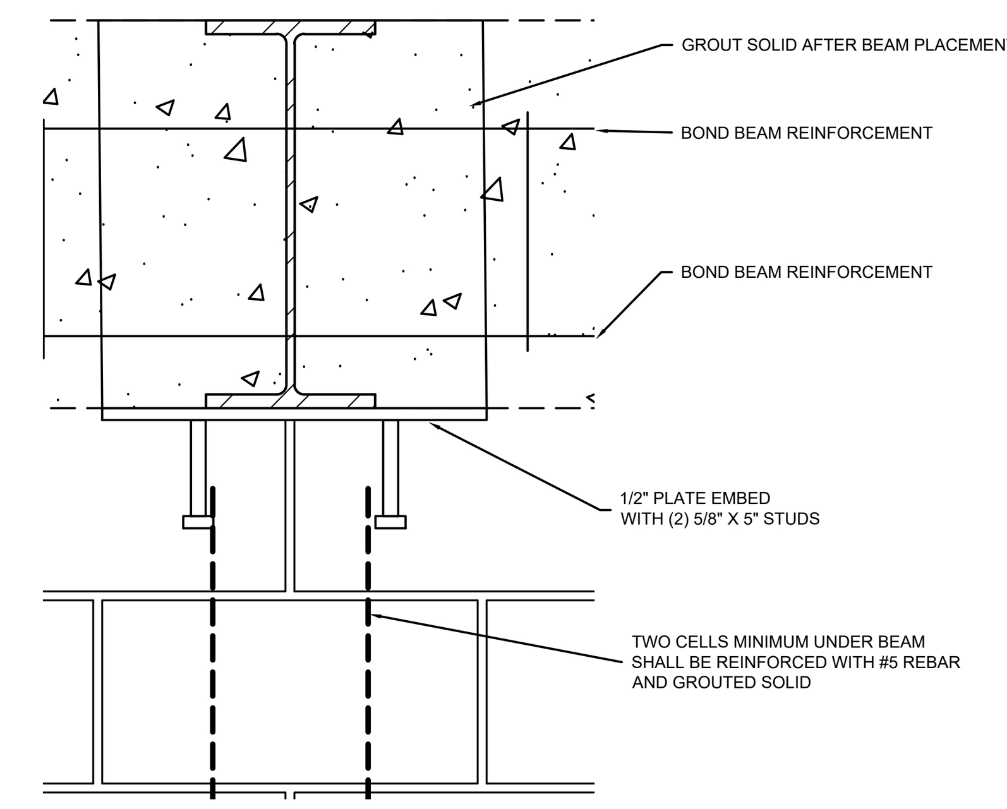


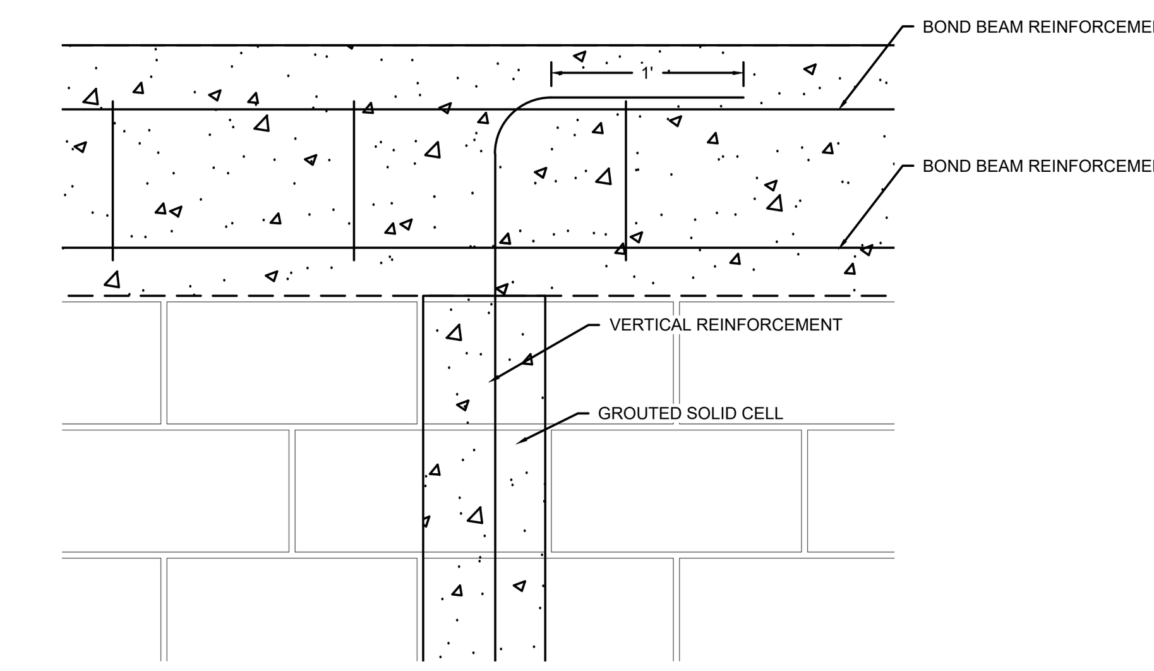
D-1 / S-11 EAVE DETAIL
SCALE: 1/2" = 1'-0"



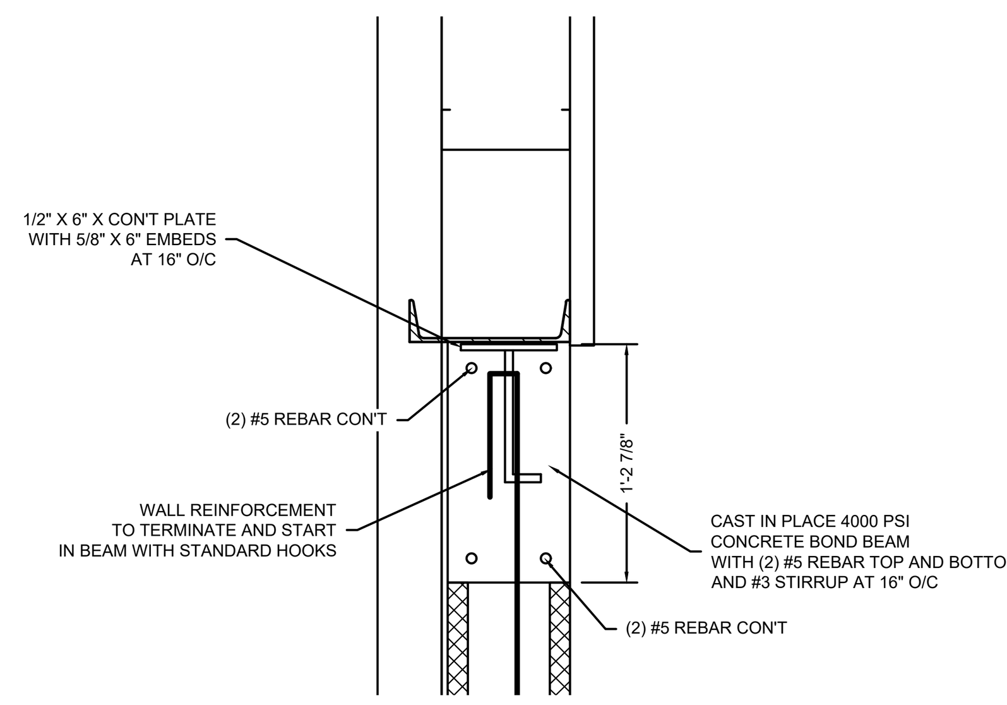
D-2 / S-11 WALL DETAIL
SCALE: 1/2" = 1'-0"



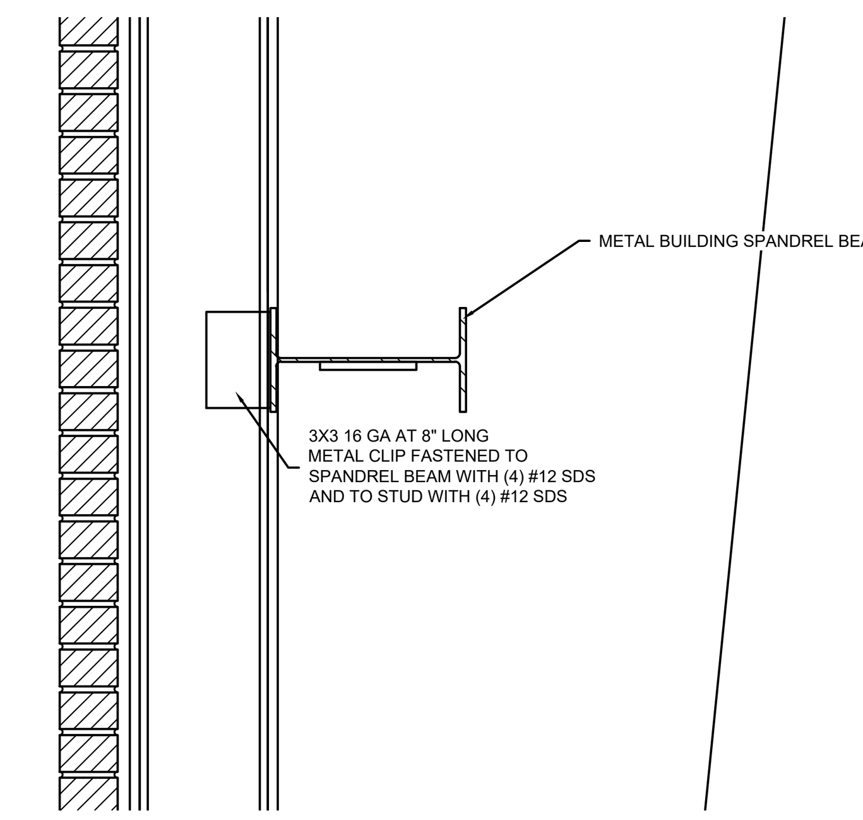
MP-1 / S-11 MASONRY POCKET DETAIL
SCALE: 1 1/2" = 1'-0"



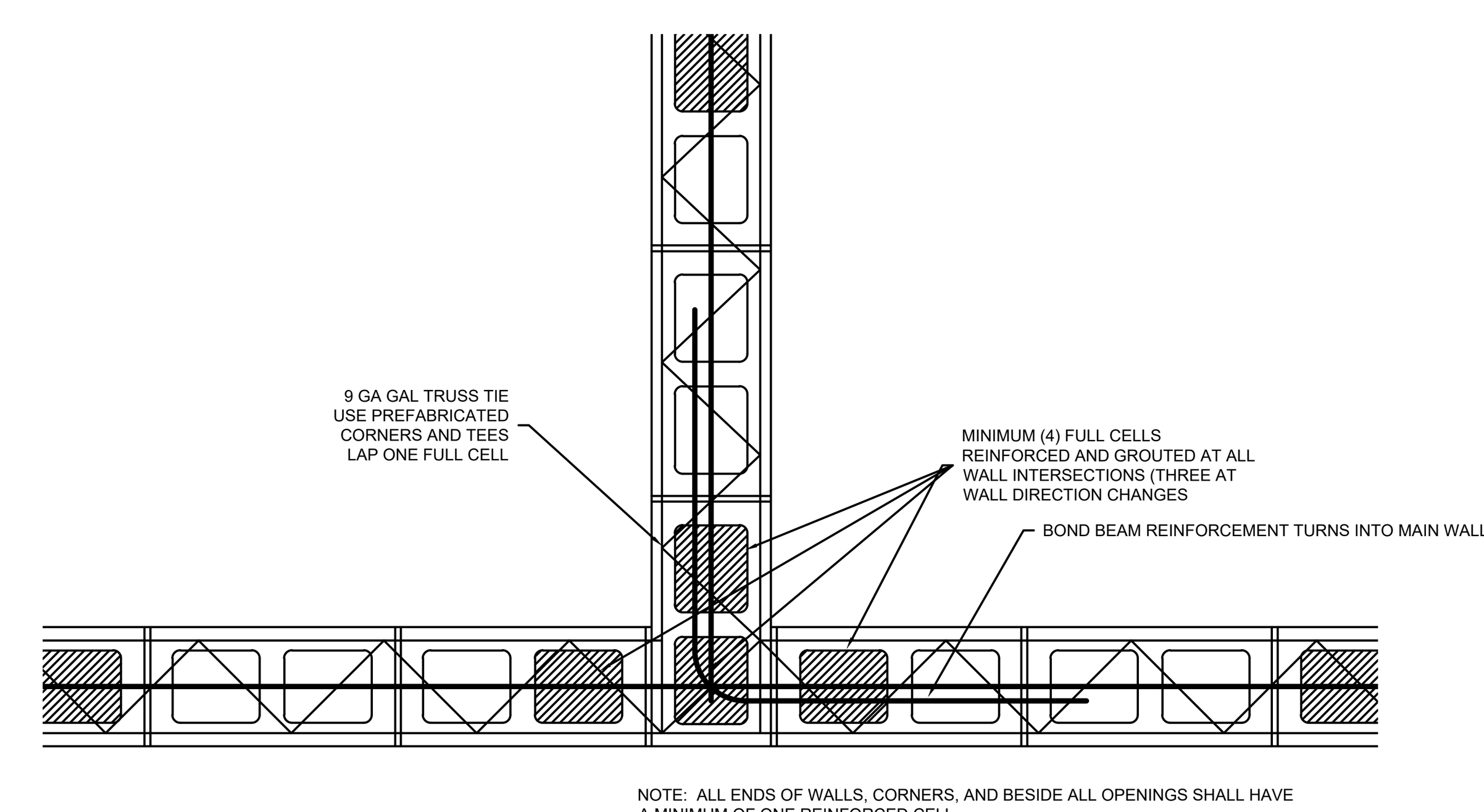
MT-1 / S-11 BOND BEAM VERTICAL BAR TERMINATION DETAIL
SCALE: 1" = 1'-0"



BB-1 / S-11 BOND BEAM FIRST FLOOR (INT. AND EXT.)
SCALE: 1" = 1'-0"



WSA-1 / S-11 WALL STUD ATTACHMENT DETAIL
SCALE: 1" = 1'-0"



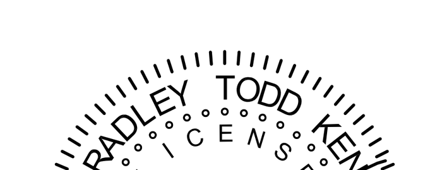
NOTE: ALL ENDS OF WALLS, CORNERS, AND BESIDE ALL OPENINGS SHALL HAVE A MINIMUM OF ONE REINFORCED CELL.

MT-2 / S-11 MASONRY WALL INTERSECTION DETAIL
SCALE: 1" = 1'-0"

MASONRY VENEER LOOSE LINTEL SCHEDULE

OPENING	LINTEL	BEARING EACH END
6' OR LESS	L4x3-1/2x1/4	8"
OVER 6' TO 10'-0"	L7x4x3/8	12"

NOTES:
1. FOR OPENINGS 6'-0" AND LARGER, PROVIDE SOLID MASONRY JAMB UNDER LINTEL EACH SIDE OF OPENING.
2. FOR OPENINGS LARGER THAN 10'-0", PROVIDE (1) 5/8"Øx1'-0" ANCHOR BOLT EACH END OF LINTEL.
3. ALL STEEL ANGLES USED FOR BRICK VENEER LOOSE LINTELS SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123.



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